

## DETERMINATION OF CBR OF SUBGRADE SOIL AT FILED DENSITY (UNSOAKED) AND SATURATED DENSITY (SOAKED) CONDITION FROM DCP TEST

VUNDAVALLI V.V SATYANARAYANA<sup>1</sup>, E. NATHANIELU<sup>2</sup>

<sup>1</sup>M. Tech Scholar, “Department of Civil engineering, ST. Johns College of engineering and Technology, Yerrakota, Yemmiganur, Kurnool, 518360, AP,India.

<sup>2</sup>Assistant Professor, “Department of Civil engineering, ST. Johns College of engineering and Technology, Yerrakota, Yemmiganur, Kurnool, 518360, AP,India.

**Abstract** IRC-37-2001, Indian Standard arrangements with the plan of adaptable asphalt and suggests the California Bearing Proportion (CBR) as a mark of subgrade soil strength. The subbase/base thickness of asphalt is administered by the CBR worth of the subgrade soil along with a few different boundaries, for example, traffic power, climatic circumstances etc. The regular CBR testing strategy is costly, tedious and its repeatability is low. Also it is truly challenging to shape the example at the ideal in-situ thickness in the research center CBR test. Upsides of in-situ thickness are under assessed because of nearby sogginess of surface water permeation and stress discharge while taking out the example. Dynamic cone entrance test (DCPT) esteem led in the field can be utilized to gauge the CBR esteem gave a reasonable relationship exists among CBR and DCPT esteem. In the current review an endeavor has been made to lay out a connection between DCPT worth and CBR.

### 1. INTRODUCTION

The plan of new adaptable asphalts and restoration of existing asphalts needs a precise assessment of CBR esteem. In the plan of overlays commonly Benkelman's shaft strategy and Falling Weight Deflectometer (FWD) are utilized however these techniques are refined and tedious. Scala (1956) has effectively utilized unique cone penetrometer (DCP) for assessing the strength of soil. The review was essentially corresponding to application in plan and fortifying of existing asphalts. A portion of the work with respect to connection among's DCPT and CBR has been accounted for in writing (Smith and Pratt 1983, Livenh 1989) yet the circumstances considered isn't reproducing the genuine expressway condition.

During the plan of new asphalts or reinforcing of existing one most horrendously awful conceivable natural circumstances to be looked by the roadway during its plan life ought to be mimicked. Thusly in situ CBR tests must be directed in the wake of soaking the current sub grades completely. Be that as it may, it is truly challenging to lead a field splashed CBR test and is practically unreasonable in numerous circumstance. Then again in the event of a research facility CBR test examples in the wake of being shaped at insitu thickness will generally give higher upsides of CBR than those got in the field particularly for sandy soils (Haison 1987). The thing that matters is because of the limiting impact of unbending mold in research center tests. Again in field CBR tests, commonly deceptive upsides of CBR is gotten, at whatever point cylinder tip lays on a little stone molecule or rock.

Keeping in view the above expressed constraints of field as well as research center CBR tests, it was chosen to direct unique cone entrance test (DCPT) instead of CBR tests. The DCPT test values can be utilized to gauge the CBR values gave a reasonable relationship exists between the CBR and the DCPT esteem. Advancement of any such relationship might turn out to be extremely compelling instrument for parkway engineers. Different advantages of the relationship are the accompanying: (a) It might help improving thruway development quality control; (b) It might help guaranteeing long haul asphalt execution and steadiness; and (c) It might help accomplishing more uniform underlying property. In the current review DCP tests were directed along the 8 km significant length of the left bank of Sidhwan channel going through the southern piece of

Ludhiana city (Punjab) for enlarging and reinforcing of the current street.

Absolute 8 areas were reserved at a timespan km subsequent to visiting the site. The span was chosen in light of consistency of soil accessible along the entire stretch. The current review portrays a progression of DCP tests led at insitu conditions and absorbed situ condition. Notwithstanding the above field test, lab splashed CBR tests shaped at insitu thickness were likewise done. In the current paper the outcomes got from the tests were introduced and examined. It is likewise critical to take note of that by practicing minimal additional consideration a few impediments of the DCP test, for example, dulling of cone because of its reshaped use and lacking fall of sledge were survived.

## 2. LITERATURE SURVEY

**Schnaid et al. (2017)** presented data showing the promising direct comparisons possible with this approach. The simplicity of the DCP test and dependence of results on user efficiency may preclude such an approach being developed for this test, however, it would be the most rigorous. Consequently, direct empirical relationships are likely to remain the state of practice for some time to come.

**Adil Mehraj et al** thought about that further field and exploration focus preliminary examinations may be engaged on the topic with the objective that unflinching nature of above proposed conditions may be upgraded and the proposed conditions may be speedily used to evaluate different black-top structure plan boundaries like in place thickness of sub-audit, undisturbed California Bearing extent, soaked California Bearing extent for the most part more complement has been set on appraisal of undisturbed CBR through DCP connections because basically the lacustrine soils in Kashmir valley are astoundingly sensitive to remolding. Such soils are for the most part consolidated and by and large showcase a laid out way. Effect of set up retaining such soil is incredibly insignificant as a result of their low vulnerability and elevated degree of association. In this manner deviant proportion of set up and un-

drenched CBR, using association, is a strong proportion of sub-survey quality, explicitly.

**Parampreet Kaur et al.** fluctuating from 52% to 66%. Nature of soil is non plastic. Quite far is fuming between 18% to 20%. In situ sogginess content lies in the extent of 2.04% to 8.69% and in situ thickness at that areas are varying from 3.89% to 8.6%. It is watched that DCPT considering CBR regards for sprinkled condition isn't however much the CBR regards obtained for soaked CBR tests. This is a result of higher constraint weight in the rigid form using as a piece of the test procedure of soaked CBR tests. The drenched CBR assessments of uniform soils which has relative ascribes can be settled quickly and will have adequate accuracy using DCP test occurs. For existing circumstances, the in situ DCPT can be coordinated for affirmation of field CBR regard for in situ thickness. Controlling quality and achieving more uniform assistant property in further developing expressway development may be valuable.

## 3. METHODOLOGY AND MATERIALS USED

### Methodology

Strategy is the efficient, hypothetical investigation of the techniques applied to a field of study. It involves the hypothetical examination of the group of strategies and standards related with a part of information.

A strategy doesn't decide to give arrangements - it is thusly, not equivalent to a technique. All things considered, a technique offers the hypothetical supporting for figuring out which strategy, set of techniques, or best practices can be applied to a particular case, for instance, to work out a particular outcome.

### Red soil

Red soil is a significant soil asset, which bears significant ramifications for practical advancement of farming and sound development of economy. We additionally summed up how the iron redox cycling might be impacted by other biogeochemical cycles or

dynamic constituents, for example, the nitrogen cycling, the sulfur cycling and humic substances. At last, future exploration needs relating to press redox cycling coupled to the destiny of weighty metals are recommended. The outcomes summed up in this survey might give experiences to settling the weighty metal contamination of paddy soils in the red soil districts.

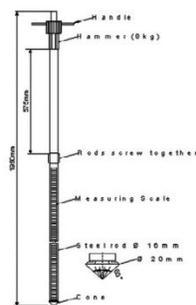


Red soil

### DCPT Test

The DCP tests were conducted according to the procedure laid down in ASTM-D6951-3 (2003). The apparatus consists of 16mm diameter steel rod in which a tempered steel cone with a 20 mm base diameter and a 60 degree point angle is attached. The DCP is driven into the soil by a 8kg hammer with a free fall of 575mm. The hammer correction factor is unity for 8kg hammer. Figure1 shows the dimensions of the dynamic cone penetrometer.

The DCP index or reading is defined as the penetration depth (D) in mm for a single drop of hammer. The cone is driven in to the ground upto the desired depth and average DCP index is calculated for a single blow. Depth of penetration considered in the study was 800mm because the stresses induced due to the wheel load becomes negligible beyond this depth.



DCP test setup

Experimental tests to be conducted on soil

### Soaked soil/ Un soaked soil

1. Atterberg's limits
2. Compaction
3. Un confined compressive strength
4. CBR test
5. DCPT Test

## 4. RESULTS AND ANALYSIS

### Atterberg's limits

Liquid limit for un Soaked soil

Observations	Sample 1	Sample 2	Sample 3
No. Of blows	25	50	60
Mass of can (g) (W <sub>1</sub> )	36	38	36
Mass of can + wet soil (g) (W <sub>2</sub> )	41.5	42.2	44.5
Mass of can + dry soil (g) (W <sub>3</sub> )	40	41.5	42.5
Water content	37.5	20	30.76

Liquid limit for soaked soil

Observations	Sample 1	Sample 2	Sample 3
No. Of blows	15	21	25
Mass of can (g) (W <sub>1</sub> )	36	36	36
Mass of can + wet soil (g) (W <sub>2</sub> )	44.4	42.8	44
Mass of can + dry soil (g) (W <sub>3</sub> )	47.5	46.2	48.6
Water content	73	66.7	63.5

### Plastic limit

Plastic limit for un Soaked soil

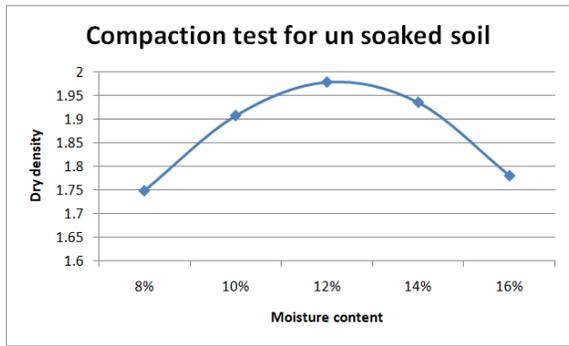
Observations	Sample 1	Sample 2
Mass of can + wet soil (g)	55.1	59.2
Mass of can + dry soil (g)	54	58
Water content (%)	27.5	30
Average water content (%)	28.75%	

Plastic limit for soaked soil

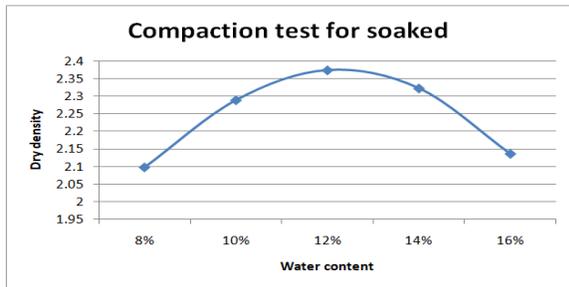
Observations	Sample 1	Sample 2
Mass of can + wet soil (g)	39.5	46
Mass of can + dry soil (g)	38	43
Water content (%)	50	42.85
Average water content (%)	46.425	

Compaction test

Un soaked soil

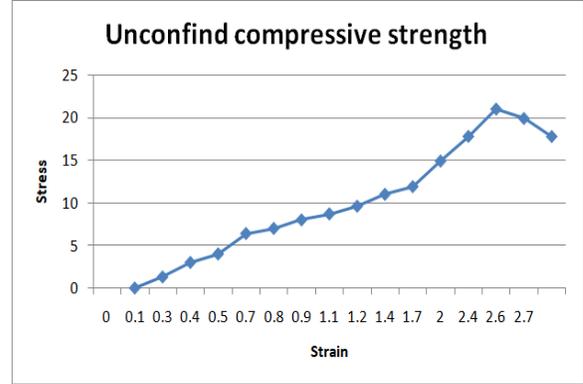
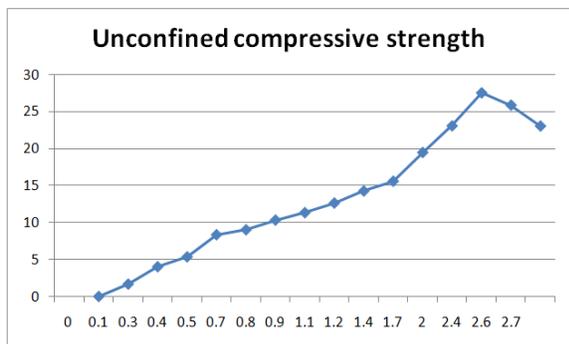


Soaked soil



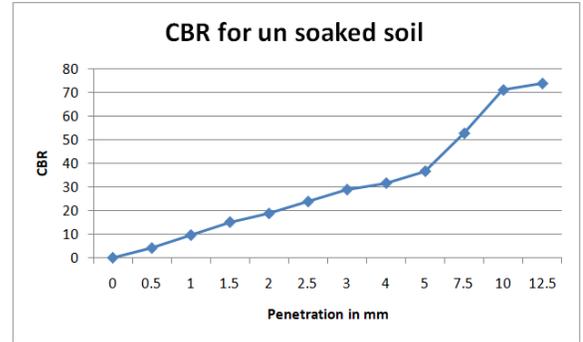
Unconfined compressive strength

Un soaked soil

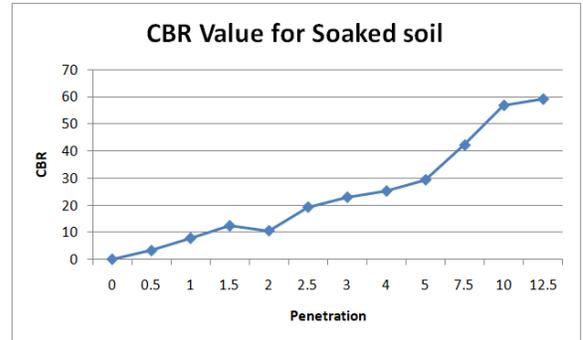


CBR Test

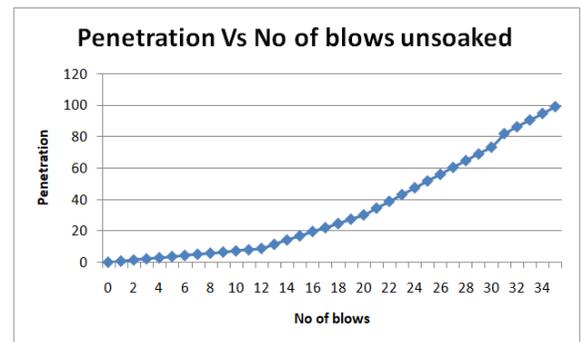
Un soaked soil



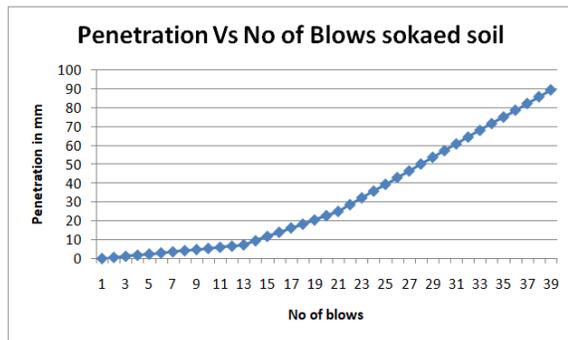
Soaked soil



DCPT Test for un soaked soil



### DCPT Test for soaked soil



## 5. CONCLUSIONS

Based on the study, following conclusions can be drawn:

1. The CBR value of uniform soils having similar characteristics can be determined quickly and with adequate accuracy using the DCPT results.
2. Once the correlation is established between CBR index for tests conducted under different conditions and compaction level or in-situ density.
3. The soaked CBR value in the field can be determined very quickly by conducting the in-situ DCPT for existing conditions and using the CBRI value for that particular condition.
4. For construction of new embankments or strengthening of existing pavements, DCPT will be a very useful tool for evaluating the strength of sub grade in terms of CBR value.
5. It may helpful in enhancing highway construction quality control, ensuring long-term pavement performance, stability and achieving more uniform structural property.

## REFERENCES

[1]. Samuel I.K. Ampadu, D. Dzitse-Awuku, et al.,(2019), "Model tests for bearing capacity in a lateritic soil and implications for the use of the dynamic cone penetrometer", Proceedings of the 17th International Conference on Soil Mechanics and

Geotechnical Engineering M. Hamza et al. (Eds.) © 2009 IOS Press

[2]. Er. Sobiya Sidiq, Dr. Rajesh Goyal, et al.,(2016), "EXPERIMENTAL STUDY ON DEVELOPMENT OF CORRELATION BETWEEN CBR AND DYNAMIC CONE PENETROMETER TEST" International Research Journal of Engineering and Technology (IRJET) e-ISSN: 2395 -0056 Volume: 03 Issue: 11 | Nov -2016

[3]. Deepika.Chukka, Chakravarthi.V.K, et al.,(2012), "Evaluation of Properties of Soil Subgrade Using Dynamic Cone Penetration Index – A Case Study", International Journal of Engineering Research and Development e-ISSN: 2278-067X, p-ISSN: 2278-800X Volume 4, Issue 4 (October 2012), PP. 07-15

[4]. M. M. E. Zumrawi-"Prediction of In-situ CBR of Subgrade Cohesive Soils from Dynamic Cone Penetromete and Soil Properties- IACSIT International Journal of Engineering and Technology, Vol. 6, No. 5, October 2014.

[5]. K.A.K.Karunaprema and A.G.H.J.Edirisinghe-"A Laboratory Investigation on the Relationship between Dynamic Cone Penetrometer Value and Soaked California Bearing Ratio"-Annual Transactions of IESL -2003 section (Digesr), pp 120-122, Institution of engineers, Sri Lank

[6]. P. K. SAHOO & K. SUDHAKAR REDDY-"Evaluation of subgrade soil using Dynamic cone Penetrometer"- 384International Journal of Earth Sciences and Engineering ISSN 0974-5904, Vol. 02, No. 04, August 2009, pp. 384-388

[7]. Deepika Chukka, Chakravarthi.V.K-"Evaluation of Properties of Soil Subgrade Using Dynamic Cone Penetration Index"- International Journal of Engineering Research and Development e-ISSN: 2278-067X, p-ISSN: 2278-800X, www.ijerd.com Volume 4, Issue 4 (October 2012), PP. 07-15 7

[8]. Adil Mehraj , Firddous Ahmed, Mohammad Zubair, Irshad Gani Bhat & Ajaz Masood\_ "Experimental Study On Insitu and

- Laboratory Co-Relation of Dynamic Cone Penetrometer Test Result With Sub- Grade CBR for Lacustrine and Alluvial Plain Soil of Kashmir"- International Journal of Civil Structural, Environment and Infrastructure (IJCSSEI) ISSN(P): 2249-6866;ISSN(E): 2249-7978 vol. 4,Issue 4, Aug 2014, 45-52 TJPRC pvt.Ltd.
- [9]. Parampreet Kaur, K.S.Gill, B.S.Walia-"Correlation Between Soaked CBR Value and CBR Value Obtained With Dynamic Cone Penetrometer"-IJREAS Volume2, Issue 2 (February 2012) ISSN:2249-3905.
- [10]. ASTM-D 6951-3 (2003). Standard Test Method for Use of the Dynamic Cone Penetrometer in Shallow Pavement Applications.
- [11]. Black, W.P.M. (1961). The Calculation of Laboratory and In situ Values of California Bearing Ratio from Bearing Capacity Data. *Geotechnique*, 11, 14-21.
- [12]. Choudhary, A.K., Jha, J.N. & Gill, K.S. (2010). Utilization of Plastic Waste for Improving the Subgrades in Flexible Pavements. *Geotechnical Special Publication*, ASCE, 203,320-326.
- [13]. Haison, J.A. (1987). Correlation Between California Bearing Ratio and Dynamic Cone Penetrometer Strength Measurement of Soils. *Proceeding, Institution of civil engineering*, 83(2), 833-844.
- [14]. Livneh, M. (1989). Validation of Correlations Between a Number of Penetrating Tests and In situ California Tests. *Transport Research Record*, 1219, 56-67.
- [15]. Scala, A. J. (1956). Simple Methods of Flexible Pavement Design Using Cone Penetrometer. *N.Z.Eng.*, 11(2).
- [16]. Smith, R.B and Pratt D.N. (1983). A Field Study of In situ California Bearing Ratio and Dynamic Cone Penetrometer Testing for Road Investigations. *Australian Road Research*, 13(4), 285-294.
- [17]. Webster, S.L, Grau, R.H. & Williams T.P. (1992) Description and Application of Dual Mass Dynamic Cone Penetration Report. GL-93-3. Department of Army, Washington DC, 19.
- [18]. Sawangsurriya, A., Wachiraporn, S. & Sramoon, W. (2008). Assessment of Modulus and CBR of Pavement Materials for Design and Construction. Seminar: Bureau of Materials-Analysis and Inspection of Highways, Bangkok, Thailand, 31-50.